CHAPTER 17
STRUCTURAL TESTS AND SPECIAL INSPECTIONS

SECTION 1701
GENERAL
1701.1 Scope. The provisions of this chapter shall govern the quality, workmanship and requirements for materials covered. Materials of construction and tests shall conform to the applicable standards listed in this code.

1701.2 New materials. New building materials, equipment, appliances, systems or methods of construction not provided for in this code, and any material of questioned suitability proposed for use in the construction of a building or structure, shall be subject to the tests prescribed in this chapter and in the approved rules to determine character, quality and limitations of use.

1701.3 Used materials. The use of second-hand materials that meet the minimum requirements of this code for new materials shall be permitted.

SECTION 1702
DEFINITIONS
1702.1 General. The following words and terms shall, for the purposes of this chapter and as used elsewhere in this code, have the meanings shown herein.

APPROVED AGENCY. An established and recognized agency regularly engaged in conducting tests or furnishing inspection services, when such agency has been approved.

APPROVED FABRICATOR. An established and qualified person, firm or corporation approved by the building official pursuant to Chapter 17 of this code.

CERTIFICATE OF COMPLIANCE. A certificate stating that materials and products meet specified standards or that work was done in compliance with approved construction documents.

DESIGNATED SEISMIC SYSTEM. Those architectural, electrical and mechanical systems and their components that require design in accordance with Chapter 13 of ASCE 7 and for which the component importance factor, Ip, is greater than 1 in accordance with Section 13.1.3 of ASCE 7.

FABRICATED ITEM. Structural, load-bearing or lateral load-resisting assemblies consisting of materials assembled prior to installation in a building or structure, or subjected to operations such as heat treatment, thermal cutting, cold working or reforming after manufacture and prior to installation in a building or structure. Materials produced in accordance with standard specifications referenced by this code, such as rolled structural steel shapes, steel-reinforcing bars, masonry units, and wood structural panels or in accordance with a standard, listed in Chapter 35, which provides requirements for quality control done under the supervision of a third-party quality control agency shall not be considered “fabricated items.”

INSPECTION CERTIFICATE. An identification applied on a product by an approved agency containing the name of the manufacturer, the function and performance characteristics, and the name and identification of an approved agency that indicates that the product or material has been inspected and evaluated by an approved agency (see Section 1703.5 and “Label,” “Manufacturer’s designation” and “Mark”).

INTUMESCENT FIRE-RESISTANT COATINGS. Thin film liquid mixture applied to substrates by brush, roller, spray or trowel which expands into a protective foamed layer to provide fire-resistant protection of the substrates when exposed to flame or intense heat.

MAIN WINDFORCE-RESISTING SYSTEM. An assemblage of structural elements assigned to provide support and stability for the overall structure. The system generally receives wind loading from more than one surface.

MASTIC FIRE-RESISTANT COATINGS. Liquid mixture applied to a substrate by brush, roller, spray or trowel that provides fire-resistant protection of a substrate when exposed to flame or intense heat.

SPECIAL INSPECTION. Inspection as herein required of the materials, installation, fabrication, erection or placement of components and connections requiring special expertise to ensure compliance with approved construction documents and referenced standards (see Section 1704).

SPECIAL INSPECTION, CONTINUOUS. The full-time observation of work requiring special inspection by an approved special inspector who is present in the area where the work is being performed.

SPECIAL INSPECTION, PERIODIC. The part-time or intermittent observation of work requiring special inspection by an approved special inspector who is present in the area where the work has been or is being performed and at the completion of the work.

SPRAYED FIRE-RESISTANT MATERIALS. Cementitious or fibrous materials that are sprayed to provide fire-resistant protection of the substrates.

STRUCTURAL OBSERVATION. The visual observation of the structural system by a registered design professional for general conformance to the approved construction documents. Structural observation does not include or waive the responsibility for the inspection required by Section 110, 1704 or other sections of this code.

SECTION 1703
APPROVALS
1703.1 Approved agency. An approved agency shall provide all information as necessary for the building official to determine that the agency meets the applicable requirements.

1703.1.1 Independence. An approved agency shall be objective, competent and independent from the contractor responsible for the work being inspected. The agency shall
also disclose possible conflicts of interest so that objectivity can be confirmed.

1703.1.2 Equipment. An approved agency shall have adequate equipment to perform required tests. The equipment shall be periodically calibrated.

1703.1.3 Personnel. An approved agency shall employ experienced personnel educated in conducting, supervising and evaluating tests and/or inspections.

1703.2 Written approval. Any material, appliance, equipment, system or method of construction meeting the requirements of this code shall be approved in writing after satisfactory completion of the required tests and submission of required test reports.

1703.3 Approved record. For any material, appliance, equipment, system or method of construction that has been approved, a record of such approval, including the conditions and limitations of the approval, shall be kept on file in the building official’s office and shall be open to public inspection at appropriate times.

1703.4 Performance. Specific information consisting of test reports conducted by an approved testing agency in accordance with standards referenced in Chapter 35, or other such information as necessary, shall be provided for the building official to determine that the material meets the applicable code requirements.

1703.4.1 Research and investigation. Sufficient technical data shall be submitted to the building official to substantiate the proposed use of any material or assembly. If it is determined that the evidence submitted is satisfactory proof of performance for the use intended, the building official shall approve the use of the material or assembly subject to the requirements of this code. The costs, reports and investigations required under these provisions shall be paid by the applicant.

1703.4.2 Research reports. Supporting data, where necessary to assist in the approval of materials or assemblies not specifically provided for in this code, shall consist of valid research reports from approved sources.

1703.5 Labeling. Where materials or assemblies are required by this code to be labeled, such materials and assemblies shall be labeled by an approved agency in accordance with Section 1703. Products and materials required to be labeled shall be labeled in accordance with the procedures set forth in Sections 1703.5.1 through 1703.5.3.

1703.5.1 Testing. An approved agency shall test a representative sample of the product or material being labeled to the relevant standard or standards. The approved agency shall maintain a record of the tests performed. The record shall provide sufficient detail to verify compliance with the test standard.

1703.5.2 Inspection and identification. The approved agency shall periodically perform an inspection, which shall be in-plant if necessary, of the product or material that is to be labeled. The inspection shall verify that the labeled product or material is representative of the product or material tested.

1703.5.3 Label information. The label shall contain the manufacturer’s or distributor’s identification, model number, serial number or definitive information describing the product or material’s performance characteristics and approved agency’s identification.

1703.6 Evaluation and follow-up inspection services. Where structural components or other items regulated by this code are not visible for inspection after completion of a prefabricated assembly, the applicant shall submit a report of each prefabricated assembly. The report shall indicate the complete details of the assembly, including a description of the assembly and its components, the basis upon which the assembly is being evaluated, test results and similar information and other data as necessary for the building official to determine conformance to this code. Such a report shall be approved by the building official.

1703.6.1 Follow-up inspection. The applicant shall provide for special inspections of fabricated items in accordance with Section 1704.2.

1703.6.2 Test and inspection records. Copies of necessary test and inspection records shall be filed with the building official.

SECTION 1704
SPECIAL INSPECTIONS

1704.1 General. Where application is made for construction as described in this section, the owner or the registered design professional in responsible charge acting as the owner’s agent shall employ one or more approved agencies to perform inspections during construction on the types of work listed under Section 1704. These inspections are in addition to the inspections identified in Section 110.

The special inspector shall be a qualified person who shall demonstrate competence, to the satisfaction of the building official, for the inspection of the particular type of construction or operation requiring special inspection. The registered design professional in responsible charge and engineers of record involved in the design of the project are permitted to act as the approved agency and their personnel are permitted to act as the special inspector for the work designed by them, provided those personnel meet the qualification requirements of this section to the satisfaction of the building official. The special inspector shall provide written documentation to the building official demonstrating his or her competence and relevant experience or training. Experience or training shall be considered relevant when the documented experience or training is related in complexity to the same type of special inspection activities for projects of similar complexity and material qualities. These qualifications are in addition to qualifications specified in other sections of this code.

Exceptions:

1. Special inspections are not required for work of a minor nature or as warranted by conditions in the jurisdiction as approved by the building official.

2. Special inspections are not required for building components unless the design involves the practice of professional engineering or architecture as defined by applicable state statutes and regulations governing
the professional registration and certification of engineers or architects.

3. Unless otherwise required by the building official, special inspections are not required for Group U occupancies that are accessory to a residential occupancy including, but not limited to, those listed in Section 312.1.

1704.1.1 Statement of special inspections. The applicant shall submit a statement of special inspections prepared by the registered design professional in responsible charge in accordance with Section 107.1 as a condition for issuance. This statement shall be in accordance with Section 1705.

Exceptions:

1. A statement of special inspections is not required for structures designed and constructed in accordance with the conventional construction provisions of Section 2308.

2. The statement of special inspections is permitted to be prepared by a qualified person approved by the building official for construction not designed by a registered design professional.

1704.1.2 Report requirement. Special inspectors shall keep records of inspections. The special inspector shall furnish inspection reports to the building official, and to the registered design professional in responsible charge. Reports shall indicate that work inspected was or was not completed in conformance to approved construction documents. Discrepancies shall be brought to the immediate attention of the contractor for correction. If they are not corrected, the discrepancies shall be brought to the attention of the building official and to the registered design professional in responsible charge prior to the completion of that phase of the work. A final report documenting required special inspections and correction of any discrepancies noted in the inspections shall be submitted at a point in time agreed upon prior to the start of work by the applicant and the building official.

1704.2 Inspection of fabricators. Where fabrication of structural load-bearing members and assemblies is being performed on the premises of a fabricator’s shop, special inspection of the fabricated items shall be required by this section and as required elsewhere in this code.

1704.2.1 Fabrication and implementation procedures. The special inspector shall verify that the fabricator maintains detailed fabrication and quality control procedures that provide a basis for inspection control of the workmanship and the fabricator’s ability to conform to approved construction documents and referenced standards. The special inspector shall review the procedures for completeness and adequacy relative to the code requirements for the fabricator’s scope of work.

Exception: Special inspections as required by Section 1704.2 shall not be required where the fabricator is approved in accordance with Section 1704.2.2.

1704.2.2 Fabricator approval. Special inspections required by Section 1704 are not required where the work is done on the premises of a fabricator registered and approved to perform such work without special inspection. Approval shall be based upon review of the fabricator’s written procedural and quality control manuals and periodic auditing of fabrication practices by a nationally recognized accrediting authority. At completion of fabrication, the approved fabricator shall submit a certificate of compliance to the building official stating that the work was performed in accordance with the approved construction documents.

1704.3 Steel construction. The special inspections for steel elements of buildings and structures shall be as required by Section 1704.3 and Table 1704.3.

Exceptions:

1. Special inspection of the steel fabrication process shall not be required where the fabricator does not perform any welding, thermal cutting or heating operation of any kind as part of the fabrication process. In such cases, the fabricator shall be required to submit a detailed procedure for material control that demonstrates the fabricator’s ability to maintain suitable records and procedures such that, at any time during the fabrication process, the material specification, grade and mill test reports for the main stress-carrying elements are capable of being determined.

2. The special inspector need not be continuously present during welding of the following items, provided the materials, welding procedures and qualifications of welders are verified prior to the start of the work; periodic inspections are made of the work in progress and a visual inspection of all welds is made prior to completion or prior to shipment of shop welding.

2.1. Single-pass fillet welds not exceeding \( \frac{1}{16} \) inch (7.9 mm) in size.

2.2. Floor and roof deck welding.

2.3. Welded studs when not installed with an automatically timed stud welding machine in accordance with Section 7 of AWS D1.1.

2.4. Welded sheet steel for cold-formed steel members.

2.5. Welding of stairs and railing systems.

3. For welded studs installed with an automatically timed stud welding machine and in accordance with Section 7 of AWS D1.1; the special inspector need not be continuously present during installation of welded studs subject to the following provisions:

3.1. The special inspector shall perform a visual inspection of all welded studs in accordance with Sections 7 and 7.8.1 of AWS D1.1. Visual inspection of welded studs installed with an automatically timed stud welding machine may take place either in the fabrication shop prior to completion or prior to shipment, or on-site prior to coverage.

3.2. The fabricator shall submit a certificate of compliance to the building official stating that the work was performed in accordance with the approved construction documents.
### TABLE 1704.3
**REQUIRED VERIFICATION AND INSPECTION OF STEEL CONSTRUCTION**

<table>
<thead>
<tr>
<th>VERIFICATION AND INSPECTION</th>
<th>CONTINUOUS</th>
<th>PERIODIC</th>
<th>REFERENCED STANDARD$^a$</th>
<th>IBC REFERENCE</th>
</tr>
</thead>
<tbody>
<tr>
<td>1. Material verification of high-strength bolts, nuts and washers:</td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>a. Identification markings to conform to ASTM standards specified in the approved construction documents.</td>
<td>—</td>
<td>X</td>
<td>AISC 360, Section A3.3 and applicable ASTM material standards</td>
<td></td>
</tr>
<tr>
<td>b. Manufacturer’s certificate of compliance required.</td>
<td>—</td>
<td>X</td>
<td>—</td>
<td>—</td>
</tr>
<tr>
<td>2. Inspection of high-strength bolting:</td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>a. Snug-tight joints.</td>
<td>—</td>
<td>X</td>
<td>AISC 360, Section M2.5</td>
<td>1704.3.3</td>
</tr>
<tr>
<td>b. Pretensioned and slip-critical joints using turn-of-nut with matchmarking, twist-off bolt or direct tension indicator methods of installation.</td>
<td>—</td>
<td>X</td>
<td>—</td>
<td>—</td>
</tr>
<tr>
<td>c. Pretensioned and slip-critical joints using turn-of-nut without matchmarking or calibrated wrench methods of installation.</td>
<td>X</td>
<td>—</td>
<td>—</td>
<td>—</td>
</tr>
<tr>
<td>3. Material verification of structural steel and cold-formed steel deck:</td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>a. For structural steel, identification markings to conform to AISC 360.</td>
<td>—</td>
<td>X</td>
<td>AISC 360, Section M5.5</td>
<td></td>
</tr>
<tr>
<td>b. For other steel, identification markings to conform to ASTM standards specified in the approved construction documents.</td>
<td>—</td>
<td>X</td>
<td>Applicable ASTM material standards</td>
<td></td>
</tr>
<tr>
<td>c. Manufacturer’s certified test reports.</td>
<td>—</td>
<td>X</td>
<td>—</td>
<td>—</td>
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<tr>
<td>4. Material verification of weld filler materials:</td>
<td></td>
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<td></td>
<td></td>
</tr>
<tr>
<td>a. Identification markings to conform to AWS specification in the approved construction documents.</td>
<td>—</td>
<td>X</td>
<td>AISC 360, Section A3.5 and applicable AWS A5 documents</td>
<td>—</td>
</tr>
<tr>
<td>b. Manufacturer’s certificate of compliance required.</td>
<td>—</td>
<td>X</td>
<td>—</td>
<td>—</td>
</tr>
<tr>
<td>5. Inspection of welding:</td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>a. Structural steel and cold-formed steel deck:</td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>1) Complete and partial joint penetration groove welds.</td>
<td>X</td>
<td>—</td>
<td></td>
<td>AWS D1.1</td>
</tr>
<tr>
<td>2) Multipass fillet welds.</td>
<td>X</td>
<td>—</td>
<td></td>
<td>1704.3.1</td>
</tr>
<tr>
<td>3) Single-pass fillet welds &gt; $\frac{5}{16}''$</td>
<td>X</td>
<td>—</td>
<td></td>
<td></td>
</tr>
<tr>
<td>4) Plug and slot welds.</td>
<td>X</td>
<td>—</td>
<td>—</td>
<td>—</td>
</tr>
<tr>
<td>5) Single-pass fillet welds $\leq \frac{5}{16}''$</td>
<td>—</td>
<td>X</td>
<td>—</td>
<td>—</td>
</tr>
<tr>
<td>6) Floor and roof deck welds.</td>
<td>—</td>
<td>X</td>
<td>—</td>
<td>AWS D1.3</td>
</tr>
</tbody>
</table>

(continued)
1704.3.1 Welding. Welding inspection and welding inspector qualification shall be in accordance with this section.

1704.3.1.1 Structural steel. Welding inspection and welding inspector qualification for structural steel shall be in accordance with AWS D1.1.

1704.3.1.2 Cold-formed steel. Welding inspection and welding inspector qualification for cold-formed steel floor and roof decks shall be in accordance with AWS D1.3.

1704.3.1.3 Reinforcing steel. Welding inspection and welding inspector qualification for reinforcing steel shall be in accordance with AWS D1.4 and ACI 318.

1704.3.2 Details. The special inspector shall perform an inspection of the steel frame to verify compliance with the details shown on the approved construction documents, such as bracing, stiffening, member locations and proper application of joint details at each connection.

1704.3.3 High-strength bolts. Installation of high-strength bolts shall be in accordance with AISC 360.

1704.3.3.1 General. While the work is in progress, the special inspector shall determine that the requirements for bolts, nuts, washers and paint; bolted parts and installation and tightening in such standards are met. For bolts requiring pretensioning, the special inspector shall observe the preinstallation testing and calibration procedures when such procedures are required by the installation method or by project plans or specifications; determine that all plies of connected materials have been drawn together and properly snugged and monitor the installation of bolts to verify that the selected procedure for installation is properly used to tighten bolts. For joints required to be tightened only to the snug-tight condition, the special inspector need only verify that the connected materials have been drawn together and properly snugged.

1704.3.3.2 Periodic monitoring. Monitoring of bolt installation for pretensioning is permitted to be performed on a periodic basis when using the turn-of-nut method with matchmarking techniques, the direct tension indicator method or the alternate design fastener (twist-off bolt) method. Joints designated as snug tight need be inspected only on a periodic basis.

1704.3.3.3 Continuous monitoring. Monitoring of bolt installation for pretensioning using the calibrated wrench method or the turn-of-nut method without matchmarking shall be performed on a continuous basis.

1704.3.4 Cold-formed steel trusses spanning 60 feet or greater. Where a cold-formed steel truss clear span is 60 feet (18 288 mm) or greater, the special inspector shall verify that the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing are installed in accordance with the approved truss submittal package.

1704.4 Concrete construction. The special inspections and verifications for concrete construction shall be as required by this section and Table 1704.4.

For SI: 1 inch = 25.4 mm.

a. Where applicable, see also Section 1707.1, Special inspection for seismic resistance.

### TABLE 1704.3—continued

#### REQUIRED VERIFICATION AND INSPECTION OF STEEL CONSTRUCTION

<table>
<thead>
<tr>
<th>VERIFICATION AND INSPECTION</th>
<th>CONTINUOUS</th>
<th>PERIODIC</th>
<th>REFERENCED STANDARD*</th>
<th>IBC REFERENCE</th>
</tr>
</thead>
<tbody>
<tr>
<td>b. Reinforcing steel:</td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>1) Verification of weldability of reinforcing steel other than ASTM A 706.</td>
<td>—</td>
<td>X</td>
<td></td>
<td></td>
</tr>
<tr>
<td>2) Reinforcing steel resisting flexural and axial forces in intermediate and special moment frames, and boundary elements of special structural walls of concrete and shear reinforcement.</td>
<td>X</td>
<td>—</td>
<td>AWS D1.4 ACI 318: Section 3.5.2</td>
<td>—</td>
</tr>
<tr>
<td>3) Shear reinforcement.</td>
<td>X</td>
<td>—</td>
<td></td>
<td></td>
</tr>
<tr>
<td>4) Other reinforcing steel.</td>
<td>—</td>
<td>X</td>
<td></td>
<td></td>
</tr>
</tbody>
</table>

6. Inspection of steel frame joint details for compliance:

a. Details such as bracing and stiffening. — X
b. Member locations. — X

c. Application of joint details at each connection. — X

For SI: 1 inch = 25.4 mm.

a. Where applicable, see also Section 1707.1, Special inspection for seismic resistance.

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2010 OREGON STRUCTURAL SPECIALTY CODE 411
2. Continuous concrete footings supporting walls of buildings three stories or less above grade plane that are fully supported on earth or rock where:

2.1. The footings support walls of light-frame construction;
2.2. The footings are designed in accordance with Table 1809.7; or
2.3. The structural design of the footing is based on a specified compressive strength, $f_c$, no greater than 2,500 pounds per square inch (psi) (17.2 MPa), regardless of the compressive strength specified in the construction documents or used in the footing construction.

3. Nonstructural concrete slabs supported directly on the ground, including prestressed slabs on grade, where the effective prestress in the concrete is less than 150 psi (1.03 MPa).

4. Concrete foundation walls constructed in accordance with Table 1807.1.6.2.

5. Concrete patios, driveways and sidewalks, on grade.

### TABLE 1704.4

#### REQUIRED VERIFICATION AND INSPECTION OF CONCRETE CONSTRUCTION

<table>
<thead>
<tr>
<th>VERIFICATION AND INSPECTION</th>
<th>CONTINUOUS</th>
<th>PERIODIC</th>
<th>REFERENCED STANDARD</th>
<th>IBC REFERENCE</th>
</tr>
</thead>
<tbody>
<tr>
<td>1. Inspection of reinforcing steel, including prestressing tendons, and placement.</td>
<td>—</td>
<td>X</td>
<td>ACI 318: 3.5, 7.1-7.7</td>
<td>1913.4</td>
</tr>
<tr>
<td>2. Inspection of reinforcing steel welding in accordance with Table 1704.3, Item 5b.</td>
<td>—</td>
<td>—</td>
<td>AWS D1.4 ACI 318: 3.5.2</td>
<td>—</td>
</tr>
<tr>
<td>3. Inspection of bolts to be installed in concrete prior to and during placement of concrete where allowable loads have been increased or where strength design is used.</td>
<td>X</td>
<td>—</td>
<td>ACI 318: 8.1.3, 21.2.8</td>
<td>1911.5, 1912.1</td>
</tr>
<tr>
<td>4. Inspection of anchors installed in hardened concrete.</td>
<td>—</td>
<td>X</td>
<td>ACI 318: 3.8.6, 8.1.3, 21.2.8</td>
<td>1912.1</td>
</tr>
<tr>
<td>5. Verifying use of required design mix.</td>
<td>—</td>
<td>X</td>
<td>ACI 318: Ch. 4, 5.2-5.4</td>
<td>1904.2.2, 1913.2, 1913.3</td>
</tr>
<tr>
<td>6. At the time fresh concrete is sampled to fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete.</td>
<td>X</td>
<td>—</td>
<td>ASTM C 172 ASTM C 31 ACI 318: 5.6, 5.8</td>
<td>1913.10</td>
</tr>
<tr>
<td>7. Inspection of concrete and shotcrete placement for proper application techniques.</td>
<td>X</td>
<td>—</td>
<td>ACI 318: 5.9, 5.10</td>
<td>1913.6, 1913.7, 1913.8</td>
</tr>
<tr>
<td>8. Inspection for maintenance of specified curing temperature and techniques.</td>
<td>—</td>
<td>X</td>
<td>ACI 318: 5.11-5.13</td>
<td>1913.9</td>
</tr>
<tr>
<td>a. Application of prestressing forces.</td>
<td>X</td>
<td>—</td>
<td>ACI 318: 18.20 ACI 318: 18.18.4</td>
<td>—</td>
</tr>
<tr>
<td>b. Grouting of bonded prestressing tendons in the seismic-force-resisting system.</td>
<td>—</td>
<td>—</td>
<td>—</td>
<td>—</td>
</tr>
<tr>
<td>10. Erection of precast concrete members.</td>
<td>—</td>
<td>X</td>
<td>ACI 318: Ch. 16</td>
<td>—</td>
</tr>
<tr>
<td>11. Verification of in-situ concrete strength, prior to stressing of tendons in posttensioned concrete and prior to removal of shores and forms from beams and structural slabs.</td>
<td>—</td>
<td>X</td>
<td>ACI 318: 6.2</td>
<td>—</td>
</tr>
<tr>
<td>12. Inspect formwork for shape, location and dimensions of the concrete member being formed.</td>
<td>—</td>
<td>X</td>
<td>ACI 318: 6.1.1</td>
<td>—</td>
</tr>
</tbody>
</table>

For SI: 1 inch = 25.4 mm.
a. Where applicable, see also Section 1707.1, Special inspection for seismic resistance.
1704.4.1 Materials. In the absence of sufficient data or documentation providing evidence of conformance to quality standards for materials in Chapter 3 of ACI 318, the building official shall require testing of materials in accordance with the appropriate standards and criteria for the material in Chapter 3 of ACI 318. Weldability of reinforcement, except that which conforms to ASTM A 706, shall be determined in accordance with the requirements of Section 3.5.2 of ACI 318.

1704.5 Masonry construction. Masonry construction shall be inspected and verified in accordance with the requirements of Sections 1704.5.1 through 1704.5.3, depending on the occupancy category of the building or structure.

Exception: Special inspections shall not be required for:
1. Empirically designed masonry, glass unit masonry or masonry veneer designed by Section 2109, 2110 or Chapter 14, respectively, or by Chapter 5, 6 or 7 of TMS 402/ACI 530/ASCE 5, respectively, when they are part of structures classified as Occupancy Category I, II or III in accordance with Section 1604.5.
2. Masonry foundation walls constructed in accordance with Table 1807.1.6.3(1), 1807.1.6.3(2), 1807.1.6.3(3) or 1807.1.6.3(4).
3. Masonry fireplaces, masonry heaters or masonry chimneys installed or constructed in accordance with Section 2111, 2112 or 2113, respectively.

1704.5.1 Empirically designed masonry, glass unit masonry and masonry veneer in Occupancy Category IV. The minimum special inspection program for empirically designed masonry, glass unit masonry or masonry veneer designed by Section 2109, 2110 or Chapter 14, respectively, or by Chapter 5, 6 or 7 of TMS 402/ACI 530/ASCE 5, respectively, in structures classified as Occupancy Category IV, in accordance with Section 1604.5, shall comply with Table 1704.5.1.

1704.5.2 Engineered masonry in Occupancy Category I, II or III. The minimum special inspection program for masonry designed by Section 2107 or 2108 or by chapters other than Chapter 5, 6 or 7 of TMS 402/ACI 530/ASCE 5 in structures classified as Occupancy Category IV, in accordance with Section 1604.5, shall comply with Table 1704.5.1.

1704.5.3 Engineered masonry in Occupancy Category IV. The minimum special inspection program for masonry designed by Section 2107 or 2108 or by chapters other than Chapter 5, 6 or 7 of TMS 402/ACI 530/ASCE 5 in structures classified as Occupancy Category IV, in accordance with Section 1604.5, shall comply with Table 1704.5.3.

1704.6 Wood construction. Special inspections of the fabrication process of prefabricated wood structural elements and assemblies shall be in accordance with Section 1704.2. Special inspections of site-built assemblies shall be in accordance with this section.

1704.6.1 High-load diaphragms. High-load diaphragms designed in accordance with Table 2306.2.1(2) shall be installed with special inspections as indicated in Section 1704.1. The special inspector shall inspect the wood structural panel sheathing to ascertain whether it is of the grade and thickness shown on the approved building plans. Additionally, the special inspector must verify the nominal size of framing members at adjoining panel edges, the nail or staple diameter and length, the number of fastener lines and that the spacing between fasteners in each line and at edge margins agrees with the approved building plans.

1704.6.2 Metal-plate-connected wood trusses spanning 60 feet or greater. Where a truss clear span is 60 feet (1828 mm) or greater, the special inspector shall verify that the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing are installed in accordance with the approved truss submittal package.

1704.7 Soils. Special inspections for existing site soil conditions, fill placement and load-bearing requirements shall be as required by this section and Table 1704.7. The approved geotechnical report, and the construction documents prepared by the registered design professionals shall be used to determine compliance. During fill placement, the special inspector shall determine that proper materials and procedures are used in accordance with the provisions of the approved geotechnical report.

Exception: Where Section 1803 does not require reporting of materials and procedures for fill placement, the special inspector shall verify that the in-place dry density of the compacted fill is not less than 90 percent of the maximum dry density at optimum moisture content determined in accordance with ASTM D 1557.

1704.8 Driven deep foundations. Special inspections shall be performed during installation and testing of driven deep foundation elements as required by Table 1704.8. The approved geotechnical report, and the construction documents prepared by the registered design professionals, shall be used to determine compliance.

1704.9 Cast-in-place deep foundations. Special inspections shall be performed during installation and testing of cast-in-place deep foundation elements as required by Table 1704.9. The approved geotechnical report, and the construction documents prepared by the registered design professionals, shall be used to determine compliance.

1704.10 Helical pile foundations. Special inspections shall be performed continuously during installation of helical pile foundations. The information recorded shall include installation equipment used, pile dimensions, tip elevations, final depth, final installation torque and other pertinent installation data as required by the registered design professional in responsible charge. The approved geotechnical report and the construction documents prepared by the registered design professional shall be used to determine compliance.

1704.11 Vertical masonry foundation elements. Special inspection shall be performed in accordance with Section 1704.5 for vertical masonry foundation elements.
### TABLE 1704.5.1
**LEVEL 1 REQUIRED VERIFICATION AND INSPECTION OF MASONRY CONSTRUCTION**

<table>
<thead>
<tr>
<th>VERIFICATION AND INSPECTION</th>
<th>FREQUENCY OF INSPECTION</th>
<th>REFERENCE FOR CRITERIA</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>CONTINUOUS</td>
<td>PERIODIC</td>
</tr>
<tr>
<td>1. Compliance with required inspection provisions of the construction documents and the approved submittals shall be verified.</td>
<td>—</td>
<td>X</td>
</tr>
<tr>
<td>2. Verification of $f_{m}$ and $f'_{mc}$ prior to construction except where specifically exempted by this code.</td>
<td>—</td>
<td>X</td>
</tr>
<tr>
<td>3. Verification of slump flow and VSI as delivered to the site for self-consolidating grout.</td>
<td>X</td>
<td>—</td>
</tr>
<tr>
<td>4. As masonry construction begins, the following shall be verified to ensure compliance:</td>
<td></td>
<td></td>
</tr>
<tr>
<td>a. Proportions of site-prepared mortar.</td>
<td>—</td>
<td>X</td>
</tr>
<tr>
<td>b. Construction of mortar joints.</td>
<td>—</td>
<td>X</td>
</tr>
<tr>
<td>c. Location of reinforcement, connectors, prestressing tendons and anchorages.</td>
<td>—</td>
<td>X</td>
</tr>
<tr>
<td>d. Prestressing technique.</td>
<td>—</td>
<td>X</td>
</tr>
<tr>
<td>e. Grade and size of prestressing tendons and anchorages.</td>
<td>—</td>
<td>X</td>
</tr>
<tr>
<td>5. During construction the inspection program shall verify:</td>
<td></td>
<td></td>
</tr>
<tr>
<td>a. Size and location of structural elements.</td>
<td>—</td>
<td>X</td>
</tr>
<tr>
<td>b. Type, size and location of anchors, including other details of anchorage of masonry to structural members, frames or other construction.</td>
<td>—</td>
<td>X</td>
</tr>
<tr>
<td>c. Specified size, grade and type of reinforcement, anchor bolts, prestressing tendons and anchorages.</td>
<td>—</td>
<td>X</td>
</tr>
<tr>
<td>d. Welding of reinforcing bars.</td>
<td>X</td>
<td>—</td>
</tr>
<tr>
<td>c. Preparation, construction and protection of masonry during cold weather (temperature below 40°F) or hot weather (temperature above 90°F).</td>
<td>—</td>
<td>X</td>
</tr>
<tr>
<td>f. Application and measurement of prestressing force.</td>
<td>X</td>
<td>—</td>
</tr>
</tbody>
</table>

(continued)
### NOTICE OF CORRECTION

The following text is not visible in the image:

> For SI: °C = \(\frac{(°F) - 32}{1.8}\).

### TABLE 1704.5.1—continued
**LEVEL 1 REQUIRED VERIFICATION AND INSPECTION OF MASONRY CONSTRUCTION**

<table>
<thead>
<tr>
<th>VERIFICATION AND INSPECTION</th>
<th>FREQUENCY OF INSPECTION</th>
<th>REFERENCE FOR CRITERIA</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>CONTINUOUS</td>
<td>PERIODIC</td>
</tr>
<tr>
<td>6. Prior to grouting, the following shall be verified to ensure compliance:</td>
<td></td>
<td></td>
</tr>
<tr>
<td>a. Grout space is clean.</td>
<td>—</td>
<td>X</td>
</tr>
<tr>
<td>b. Placement of reinforcement and connectors, and prestressing tendons and anchorages.</td>
<td>—</td>
<td>X</td>
</tr>
<tr>
<td>c. Proportions of site-prepared grout and prestressing grout for bonded tendons.</td>
<td>—</td>
<td>X</td>
</tr>
<tr>
<td>d. Construction of mortar joints.</td>
<td>—</td>
<td>X</td>
</tr>
<tr>
<td>7. Grout placement shall be verified to ensure compliance:</td>
<td></td>
<td></td>
</tr>
<tr>
<td>a. Grouting of prestressing bonded tendons.</td>
<td>X</td>
<td>—</td>
</tr>
<tr>
<td>8. Preparation of any required grout specimens, mortar specimens and/or prisms shall be observed.</td>
<td>—</td>
<td>X</td>
</tr>
</tbody>
</table>

For SI: °C = \(\frac{(°F) - 32}{1.8}\).

a. The specific standards referenced are those listed in Chapter 35.
# TABLE 1704.5.3
## LEVEL 2 REQUIRED VERIFICATION AND INSPECTION OF MASONRY CONSTRUCTION

<table>
<thead>
<tr>
<th>VERIFICATION AND INSPECTION</th>
<th>CONTINUOUS</th>
<th>PERIODIC</th>
<th>REFERENCE FOR CRITERIA</th>
</tr>
</thead>
<tbody>
<tr>
<td>1. Compliance with required inspection provisions of the construction documents and the approved submittals.</td>
<td>—</td>
<td>X</td>
<td>—</td>
</tr>
<tr>
<td>2. Verification of $f_c'$ and $f_{ac}$ prior to construction and for every 5,000 square feet during construction.</td>
<td>—</td>
<td>X</td>
<td>—</td>
</tr>
<tr>
<td>3. Verification of proportions of materials in premixed or preblended mortar and grout as delivered to the site.</td>
<td>—</td>
<td>X</td>
<td>—</td>
</tr>
<tr>
<td>4. Verification of slump flow and VSI as delivered to the site for self-consolidating grout.</td>
<td>X</td>
<td>—</td>
<td>—</td>
</tr>
<tr>
<td>5. The following shall be verified to ensure compliance:</td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>a. Proportions of site-prepared mortar, grout and prestressing grout for bonded tendons.</td>
<td>—</td>
<td>X</td>
<td>—</td>
</tr>
<tr>
<td>b. Placement of masonry units and construction of mortar joints.</td>
<td>—</td>
<td>X</td>
<td>—</td>
</tr>
<tr>
<td>c. Placement of reinforcement, connectors and prestressing tendons and anchorages.</td>
<td>—</td>
<td>X</td>
<td>—</td>
</tr>
<tr>
<td>d. Grout space prior to grout.</td>
<td>X</td>
<td>—</td>
<td>—</td>
</tr>
<tr>
<td>e. Placement of grout.</td>
<td>X</td>
<td>—</td>
<td>—</td>
</tr>
<tr>
<td>f. Placement of prestressing grout.</td>
<td>X</td>
<td>—</td>
<td>—</td>
</tr>
<tr>
<td>g. Size and location of structural elements.</td>
<td>—</td>
<td>X</td>
<td>—</td>
</tr>
<tr>
<td>h. Type, size and location of anchors, including other details of anchorage of masonry to structural members, frames or other construction.</td>
<td>X</td>
<td>—</td>
<td>—</td>
</tr>
<tr>
<td>i. Specified size, grade and type of reinforcement, anchor bolts, prestressing tendons and anchorages.</td>
<td>—</td>
<td>X</td>
<td>—</td>
</tr>
<tr>
<td>j. Welding of reinforcing bars.</td>
<td>X</td>
<td>—</td>
<td>—</td>
</tr>
<tr>
<td>k. Preparation, construction and protection of masonry during cold weather (temperature below 40°F) or hot weather (temperature above 90°F).</td>
<td>—</td>
<td>X</td>
<td>Sec. 2104.3, 2104.4</td>
</tr>
<tr>
<td>l. Application and measurement of prestressing force.</td>
<td>X</td>
<td>—</td>
<td>—</td>
</tr>
<tr>
<td>6. Preparation of any required grout specimens and/or prisms shall be observed.</td>
<td>X</td>
<td>—</td>
<td>Sec. 2105.2.2, 2105.3</td>
</tr>
</tbody>
</table>

For SI: °C = [(°F) - 32] / 1.8, 1 square foot = 0.0929 m².

a. The specific standards referenced are those listed in Chapter 35.
### TABLE 1704.7

**REQUIRED VERIFICATION AND INSPECTION OF SOILS**

<table>
<thead>
<tr>
<th>VERIFICATION AND INSPECTION TASK</th>
<th>CONTINUOUS DURING TASK LISTED</th>
<th>PERIODICALLY DURING TASK LISTED</th>
</tr>
</thead>
<tbody>
<tr>
<td>1. Verify materials below shallow foundations are adequate to achieve the design bearing capacity.</td>
<td>—</td>
<td>X</td>
</tr>
<tr>
<td>2. Verify excavations are extended to proper depth and have reached proper material.</td>
<td>—</td>
<td>X</td>
</tr>
<tr>
<td>3. Perform classification and testing of compacted fill materials.</td>
<td>—</td>
<td>X</td>
</tr>
<tr>
<td>4. Verify use of proper materials, densities and lift thicknesses during placement and compaction of compacted fill.</td>
<td>X</td>
<td>—</td>
</tr>
<tr>
<td>5. Prior to placement of compacted fill, observe subgrade and verify that site has been prepared properly.</td>
<td>—</td>
<td>X</td>
</tr>
</tbody>
</table>

### TABLE 1704.8

**REQUIRED VERIFICATION AND INSPECTION OF DRIVEN DEEP FOUNDATION ELEMENTS**

<table>
<thead>
<tr>
<th>VERIFICATION AND INSPECTION TASK</th>
<th>CONTINUOUS DURING TASK LISTED</th>
<th>PERIODICALLY DURING TASK LISTED</th>
</tr>
</thead>
<tbody>
<tr>
<td>1. Verify element materials, sizes and lengths comply with the requirements.</td>
<td>X</td>
<td>—</td>
</tr>
<tr>
<td>2. Determine capacities of test elements and conduct additional load tests, as required.</td>
<td>X</td>
<td>—</td>
</tr>
<tr>
<td>3. Observe driving operations and maintain complete and accurate records for each element.</td>
<td>X</td>
<td>—</td>
</tr>
<tr>
<td>4. Verify placement locations and plumbness, confirm type and size of hammer, record number of blows per foot of penetration, determine required penetrations to achieve design capacity, record tip and butt elevations and document any damage to foundation element.</td>
<td>X</td>
<td>—</td>
</tr>
<tr>
<td>5. For steel elements, perform additional inspections in accordance with Section 1704.3.</td>
<td>—</td>
<td>—</td>
</tr>
<tr>
<td>6. For concrete elements and concrete-filled elements, perform additional inspections in accordance with Section 1704.4.</td>
<td>—</td>
<td>—</td>
</tr>
<tr>
<td>7. For specialty elements, perform additional inspections as determined by the registered design professional in responsible charge.</td>
<td>—</td>
<td>—</td>
</tr>
</tbody>
</table>

### TABLE 1704.9

**REQUIRED VERIFICATION AND INSPECTION OF CAST-IN-PLACE DEEP FOUNDATION ELEMENTS**

<table>
<thead>
<tr>
<th>VERIFICATION AND INSPECTION TASK</th>
<th>CONTINUOUS DURING TASK LISTED</th>
<th>PERIODICALLY DURING TASK LISTED</th>
</tr>
</thead>
<tbody>
<tr>
<td>1. Observe drilling operations and maintain complete and accurate records for each element.</td>
<td>X</td>
<td>—</td>
</tr>
<tr>
<td>2. Verify placement locations and plumbness, confirm element diameters, bell diameters (if applicable), lengths, embedment into bedrock (if applicable) and adequate end-bearing strata capacity. Record concrete or grout volumes.</td>
<td>X</td>
<td>—</td>
</tr>
<tr>
<td>3. For concrete elements, perform additional inspections in accordance with Section 1704.4.</td>
<td>—</td>
<td>—</td>
</tr>
</tbody>
</table>

2010 Oregon Structural Specialty Code
STRUCTURAL TESTS AND SPECIAL INSPECTIONS

1704.12 Sprayed fire-resistant materials. Special inspections for sprayed fire-resistant materials applied to floor, roof and wall assemblies and structural members shall be in accordance with Sections 1704.12.1 through 1704.12.6. Special inspections shall be based on the fire-resistance design as designated in the approved construction documents. The tests set forth in this section shall be based on samplings from specific floor, roof and wall assemblies and structural members. Special inspections shall be performed after the rough installation of electrical, automatic sprinkler, mechanical and plumbing systems and suspension systems for ceilings, where applicable.

1704.12.1 Physical and visual tests. The special inspections shall include the following tests and observations to demonstrate compliance with the listing and the fire-resistance rating:

1. Condition of substrates.
2. Thickness of application.
3. Density in pounds per cubic foot (kg/m³).
5. Condition of finished application.

1704.12.2 Structural member surface conditions. The surfaces shall be prepared in accordance with the approved fire-resistance design and the written instructions of approved manufacturers. The prepared surface of structural members to be sprayed shall be inspected before the application of the sprayed fire-resistant material.

1704.12.3 Application. The substrate shall have a minimum ambient temperature before and after application as specified in the written instructions of approved manufacturers. The area for application shall be vented during and after application as required by the written instructions of approved manufacturers.

1704.12.4 Thickness. No more than 10 percent of the thickness measurements of the sprayed fire-resistant materials applied to floor, roof and wall assemblies and structural members shall be less than the thickness required by the approved fire-resistance design, but in no case less than the minimum allowable thickness required by Section 1704.12.4.1.

1704.12.4.1 Minimum allowable thickness. For design thicknesses 1 inch (25 mm) or greater, the minimum allowable individual thickness shall be the design thickness minus 1/4 inch (6.4 mm). For design thicknesses less than 1 inch (25 mm), the minimum allowable individual thickness shall be the design thickness minus 25 percent. Thickness shall be determined in accordance with ASTM E 605. Samples of the sprayed fire-resistant materials shall be selected in accordance with Sections 1704.12.4.2 and 1704.12.4.3.

1704.12.4.2 Floor, roof and wall assemblies. The thickness of the sprayed fire-resistant material applied to floor, roof and wall assemblies shall be determined in accordance with ASTM E 605, making not less than four measurements for each 1,000 square feet (93 m²) of the sprayed area in each story or portion thereof.

1704.12.4.2.1 Cellular decks. Thickness measurements shall be selected from a square area, 12 inches by 12 inches (305 mm by 305 mm) in size. A minimum of four measurements shall be made, located symmetrically within the square area.

1704.12.4.2.2 Fluted decks. Thickness measurements shall be selected from a square area, 12 inches by 12 inches (305 mm by 305 mm) in size. A minimum of four measurements shall be made, located symmetrically within the square area, including one each of the following: valley, crest and sides. The average of the measurements shall be reported.

1704.12.4.3 Structural members. The thickness of the sprayed fire-resistant material applied to structural members shall be determined in accordance with ASTM E 605. Thickness testing shall be performed on not less than 25 percent of the structural members on each floor.

1704.12.4.3.1 Beams and girders. At beams and girders thickness measurements shall be made at nine locations around the beam or girder at each end of a 12-inch (305 mm) length.

1704.12.4.3.2 Joists and trusses. At joists and trusses, thickness measurements shall be made at seven locations around the joist or truss at each end of a 12-inch (305 mm) length.

1704.12.4.3.3 Wide-flanged columns. At wide-flanged columns, thickness measurements shall be made at 12 locations around the column at each end of a 12-inch (305 mm) length.

1704.12.4.3.4 Hollow structural section and pipe columns. At hollow structural section and pipe columns, thickness measurements shall be made at a minimum of four locations around the column at each end of a 12-inch (305 mm) length.

1704.12.5 Density. The density of the sprayed fire-resistant material shall not be less than the density specified in the approved fire-resistance design. Density of the sprayed fire-resistant material shall be determined in accordance with ASTM E 605. The test samples for determining the density of the sprayed fire-resistant materials shall be selected as follows:

1. From each floor, roof and wall assembly at the rate of not less than one sample for every 2,500 square feet (232 m²) or portion thereof of the sprayed area in each story.
2. From beams, girders, trusses and columns at the rate of not less than one sample for each type of structural member for each 2,500 square feet (232 m²) of floor area or portion thereof in each story.

1704.12.6 Bond strength. The cohesive/adhesive bond strength of the cured sprayed fire-resistant material applied to floor, roof and wall assemblies and structural members shall not be less than 150 pounds per square foot (psf) (7.18 kN/m²). The cohesive/adhesive bond strength shall be determined in accordance with the field test specified in ASTM E 736 by testing in-place samples of the sprayed fire-resistant material.
material selected in accordance with Sections 1704.12.6.1 through 1704.12.6.3.

1704.12.6.1 Floor, roof and wall assemblies. The test samples for determining the cohesive/adhesive bond strength of the sprayed fire-resistant materials shall be selected from each floor, roof and wall assembly at the rate of not less than one sample for every 2,500 square feet (232 m²) of the sprayed area in each story or portion thereof.

1704.12.6.2 Structural members. The test samples for determining the cohesive/adhesive bond strength of the sprayed fire-resistant materials shall be selected from beams, girders, trusses, columns and other structural members at the rate of not less than one sample for each type of structural member for each 2,500 square feet (232 m²) of floor area or portion thereof in each story.

1704.12.6.3 Primer, paint and encapsulant bond tests. Bond tests to qualify a primer, paint or encapsulant shall be conducted when the sprayed fire-resistant material is applied to a primed, painted or encapsulated surface for which acceptable bond-strength performance between these coatings and the fire-resistant material has not been determined. A bonding agent approved by the SFRM manufacturer shall be applied to a primed, painted or encapsulated surface where the bond strengths are found to be less than required values.

1704.13 Mastic and intumescent fire-resistant coatings. Special inspections for mastic and intumescent fire-resistant coatings applied to structural elements and decks shall be in accordance with AWCI 12-B. Special inspections shall be based on the fire-resistance design as designated in the approved construction documents.

1704.14 Exterior insulation and finish systems (EIFS). Special inspections shall be required for all EIFS applications.

Exceptions:

1. Special inspections shall not be required for EIFS applications installed over a water-resistive barrier with a means of draining moisture to the exterior.
2. Special inspections shall not be required for EIFS applications installed over masonry or concrete walls.


1704.15 Special cases. Special inspections shall be required for proposed work that is, in the opinion of the building official, unusual in its nature, such as, but not limited to, the following examples:

1. Construction materials and systems that are alternatives to materials and systems prescribed by this code.
2. Unusual design applications of materials described in this code.
3. Materials and systems required to be installed in accordance with additional manufacturer’s instructions that prescribe requirements not contained in this code or in standards referenced by this code.

[F] 1704.16 Special inspection for smoke control. Smoke control systems shall be tested by a special inspector.

1704.16.1 Testing scope. The test scope shall be as follows:

1. During erection of ductwork and prior to concealment for the purposes of leakage testing and recording of device location.
2. Prior to occupancy and after sufficient completion for the purposes of pressure difference testing, flow measurements and detection and control verification.

[F] 1704.16.2 Qualifications. Special inspection agencies for smoke control shall have expertise in fire protection engineering, mechanical engineering and certification as air balancers.

1704.17 Radon mitigation inspections. Where radon mitigation systems are required by Section 1811 of this code, special inspections shall be provided per the methods and frequency prescribed in this section.

1704.17.1 Soil-gas-retarder membrane. After the subfloor preparation inspection and prior to the placement of concrete, a special inspector shall verify that conformance with all of the performance measures identified in Section 1811.2.2 for maintaining the efficacy of the soil-gas-retarder membrane have been achieved.

1704.17.2 Sealing of construction joints, penetrations, cracks, and other connections. After the placement of concrete, a special inspector shall verify that conformance with all of the performance measures identified in Section 1811.2.3.7 for maintaining the efficacy of the slab have been achieved.

SECTION 1705
STATEMENT OF SPECIAL INSPECTIONS

1705.1 General. Where special inspection or testing is required by Section 1704, 1707 or 1708, the registered design professional in responsible charge shall prepare a statement of special inspections in accordance with Section 1705 for submittal by the applicant (see Section 1704.1.1).

1705.2 Content of statement of special inspections. The statement of special inspections shall identify the following:

1. The materials, systems, components and work required to have special inspection or testing by the building official or by the registered design professional responsible for each portion of the work.
2. The type and extent of each special inspection.
3. The type and extent of each test.
4. Additional requirements for special inspection or testing for seismic or wind resistance as specified in Section 1705.3.
5. For each type of special inspection, identification as to whether it will be continuous special inspection or periodic special inspection.

1705.3 Seismic resistance. The statement of special inspections shall include seismic requirements for structures in Occupancy Categories III and IV for cases covered in Sections 1705.3.1 through 1705.3.5.

Exception: Seismic requirements are permitted to be excluded from the statement of special inspections for structures designed and constructed in accordance with the following:

1. The structure consists of light-frame construction; the design spectral response acceleration at short periods,
structural system; the design spectral response acceleration at short periods, $S_{D0}$, as determined in Section 1613.5.4, does not exceed 0.5g, and the height of the structure does not exceed 25 feet (7620 mm) above grade plane.

1705.3.1 Seismic-force-resisting systems. The seismic-force-resisting systems in structures assigned to Seismic Design Category C, D, E or F, in accordance with Section 1613.

Exception: Requirements for the seismic-force-resisting system are permitted to be excluded from the statement of special inspections for steel systems in structures assigned to Seismic Design Category C that are not specifically detailed for seismic resistance, with a response modification coefficient, $R$, of 3 or less, excluding cantilever column systems.

1705.3.2 Designated seismic systems. Designated seismic systems in structures assigned to Seismic Design Category D, E or F.

1705.3.3 Seismic Design Category C. The following additional systems and components in structures assigned to Seismic Design Category C:

1. Heating, ventilating and air-conditioning (HVAC) ductwork containing hazardous materials and anchorage of such ductwork.
2. Piping systems and mechanical units containing flammable, combustible or highly toxic materials.
3. Anchorage of electrical equipment used for emergency or standby power systems.

1705.3.4 Seismic Design Category D. The following additional systems and components in structures assigned to Seismic Design Category D:

1. Systems required for Seismic Design Category C.
2. Exterior wall panels and their anchorage.
3. Suspended ceiling systems and their anchorage.
5. Steel storage racks and their anchorage, where the importance factor is equal to 1.5 in accordance with Section 15.5.3 of ASCE 7.

1705.3.5 Seismic Design Category E or F. The following additional systems and components in structures assigned to Seismic Design Category E or F:

1. Systems required for Seismic Design Categories C and D.
2. Electrical equipment.

1705.3.6 Seismic requirements in the statement of special inspections. When Sections 1705.3 through 1705.3.5 specify that seismic requirements be included, the statement of special inspections shall identify the following:

1. The designated seismic systems and seismic-force-resisting systems that are subject to special inspections in accordance with Sections 1705.3 through 1705.3.5.

2. The additional special inspections and testing to be provided as required by Sections 1707 and 1708 and other applicable sections of this code, including the applicable standards referenced by this code.

1705.4 Wind resistance. The statement of special inspections shall include wind requirements for structures constructed in the following areas:

1. In wind Exposure Category B, where the 3-second-gust basic wind speed is 120 miles per hour (mph) (52.8 m/s) or greater.
2. In wind Exposure Category C or D, where the 3-second-gust basic wind speed is 110 mph (49 m/s) or greater.

1705.4.1 Wind requirements in the statement of special inspections. When Section 1705.4 specifies that wind requirements be included, the statement of special inspections shall identify the main wind-force-resisting systems and wind-resisting components subject to special inspections as specified in Section 1705.4.2.

1705.4.2 Detailed requirements. The statement of special inspections shall include at least the following systems and components:

1. Roof cladding and roof framing connections.
2. Wall connections to roof and floor diaphragms and framing.
3. Roof and floor diaphragm systems, including collectors, drag struts and boundary elements.
4. Vertical wind-force-resisting systems, including braced frames, moment frames and shear walls.
5. Wind-force-resisting system connections to the foundation.
6. Fabrication and installation of systems or components required to meet the impact-resistance requirements of Section 1609.1.2.

Exception: Fabrication of manufactured systems or components that have a label indicating compliance with the wind-load and impact-resistance requirements of this code.

SECTION 1706
SPECIAL INSPECTIONS FOR WIND REQUIREMENTS

1706.1 Special inspections for wind requirements. Special inspections itemized in Sections 1706.2 through 1706.4, unless exempted by the exceptions to Section 1704.1, are required for buildings and structures constructed in the following areas:

1. In wind Exposure Category B, where the 3-second-gust basic wind speed is 120 miles per hour (52.8 m/sec) or greater.
2. In wind Exposure Categories C or D, where the 3-second-gust basic wind speed is 110 mph (49 m/sec) or greater.

1706.2 Structural wood. Continuous special inspection is required during field gluing operations of elements of the main windforce-resisting system. Periodic special inspection is
required for nailing, bolting, anchoring and other fastening of components within the main windforce-resistant system, including wood shear walls, wood diaphragms, drag struts, braces and hold-downs.

**Exception:** Special inspection is not required for wood shear walls, shear panels and diaphragms, including nailing, bolting, anchoring and other fastening to other components of the main windforce-resistant system, where the fastener spacing of the sheathing is more than 4 inches (102 mm) on center.

### 1706.3 Cold-formed steel light-frame construction

Periodic special inspection is required during welding operations of elements of the main windforce-resistant system. Periodic special inspection is required for screw attachment, bolting, anchoring and other fastening of components within the main windforce-resistant system, including shear walls, braces, diaphragms, collectors (drag struts) and hold-downs.

**Exception:** Special inspection is not required for cold-formed steel light-frame shear walls, braces, diaphragms, collectors (drag struts) and hold-downs where either of the following apply:

1. The sheathing is gypsum board or fiberboard.
2. The sheathing is wood structural panel or steel sheets on only one side of the shear wall, shear panel or diaphragm assembly and the fastener spacing of the sheathing is more than 4 inches (102 mm) on center (o.c.).

### 1706.4 Wind-resistant components

Periodic special inspection is required for the following systems and components:

1. Roof cladding.
2. Wall cladding.

### SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE

#### 1707.1 Special inspections for seismic resistance

For structures in Occupancy Categories III and IV, special inspections itemized in Sections 1707.2 through 1707.9, unless exempted by the exceptions of Section 1704.1, 1705.3, or 1705.3.1, are required for the following:

1. The seismic-force-resisting systems in structures assigned to **Seismic Design Category** C, D, E or F, as determined in Section 1613.
2. Designated seismic systems in structures assigned to **Seismic Design Category** D, E or F.
3. Architectural, mechanical and electrical components in structures assigned to **Seismic Design Category** C, D, E or F that are required in Sections 1707.6 and 1707.7.

#### 1707.2 Structural steel

Special inspection for structural steel shall be in accordance with the quality assurance plan requirements of AISC 341.

**Exceptions:**

1. Special inspections of structural steel in structures assigned to **Seismic Design Category** C that are not specifically detailed for seismic resistance, with a response modification coefficient, $R$, of 3 or less, excluding cantilever column systems.
2. For ordinary moment frames, ultrasonic and magnetic particle testing of complete joint penetration groove welds are only required for demand critical welds.

#### 1707.3 Structural wood

Continuous special inspection is required during field gluing operations of elements of the seismic-force-resisting system. Periodic special inspection is required for nailing, bolting, anchoring and other fastening of components within the seismic-force-resisting system, including wood shear walls, wood diaphragms, drag struts, braces, shear panels and hold-downs.

**Exception:** Special inspection is not required for wood shear walls, shear panels and diaphragms, including nailing, bolting, anchoring and other fastening to other components of the seismic-force-resisting system, where the fastener spacing of the sheathing is more than 4 inches (102 mm) on center (o.c.).

#### 1707.4 Cold-formed steel light-frame construction

Periodic special inspection is required during welding operations of elements of the seismic-force-resisting system. Periodic special inspection is required for screw attachment, bolting, anchoring and other fastening of components within the seismic-force-resisting system, including shear walls, braces, diaphragms, collectors (drag struts) and hold-downs.

**Exception:** Special inspection is not required for cold-formed steel light-frame shear walls, braces, diaphragms, collectors (drag struts) and hold-downs where either of the following apply:

1. The sheathing is gypsum board or fiberboard.
2. The sheathing is wood structural panel or steel sheets on only one side of the shear wall, shear panel or diaphragm assembly and the fastener spacing of the sheathing is more than 4 inches (102 mm) on center (o.c.).

#### 1707.5 Storage racks and access floors

Periodic special inspection is required during the anchorage of access floors and storage racks 8 feet (2438 mm) or greater in height in structures assigned to **Seismic Design Category** D, E or F.

#### 1707.6 Architectural components

Periodic special inspection during the erection and fastening of exterior cladding, interior and exterior nonbearing walls and interior and exterior veneer in structures assigned to **Seismic Design Category** D, E or F.

**Exceptions:**

1. Special inspection is not required for exterior cladding, interior and exterior nonbearing walls and interior and exterior veneer 30 feet (9144 mm) or less in height above grade or walking surface.
2. Special inspection is not required for exterior cladding and interior and exterior veneer weighing 5 psf (24.5 N/m²) or less.
3. Special inspection is not required for interior nonbearing walls weighing 15 psf (73.5 N/m²) or less.
1707.7 Mechanical and electrical components. Special inspection for mechanical and electrical equipment shall be as follows:

1. Periodic special inspection is required during the anchorage of electrical equipment for emergency or standby power systems in structures assigned to Seismic Design Category C, D, E or F;
2. Periodic special inspection is required during the installation of anchorage of other electrical equipment in structures assigned to Seismic Design Category E or F;
3. Periodic special inspection is required during installation of the anchorage of piping systems intended to carry flammable, combustible or highly toxic contents and their associated mechanical units in structures assigned to Seismic Design Category C, D, E or F;
4. Periodic special inspection of the anchorage is required during the installation of HVAC ductwork that will contain hazardous materials in structures assigned to Seismic Design Category C, D, E or F; and
5. Periodic special inspection is required during the installation of vibration isolation systems in structures assigned to Seismic Design Category C, D, E or F where the construction documents require a nominal clearance of 1/4 inch (6.4 mm) or less between the equipment support frame and restraint.

1707.8 Designated seismic system verifications. The special inspector shall examine designated seismic systems requiring seismic qualification in accordance with Section 1708.4 and verify that the label, anchorage or mounting conforms to the certificate of compliance.

1707.9 Seismic isolation system. Periodic special inspection is required during the fabrication and installation of isolator units and energy dissipation devices that are part of the seismic isolation system.

SECTION 1708
STRUCTURAL TESTING FOR SEISMIC RESISTANCE

1708.1 Testing and qualification for seismic resistance. The testing and qualification specified in Sections 1708.2 through 1708.5, unless exempted from special inspections by the exceptions of Section 1704.1, 1705.3 or 1705.3.1 are required as follows:

1. The seismic-force-resisting systems in structures assigned to Seismic Design Category C, D, E or F, as determined in Section 1613 shall meet the requirements of Sections 1708.2 and 1708.3, as applicable.
2. Designated seismic systems in structures assigned to Seismic Design Category C, D, E or F subject to the special certification requirements of ASCE 7 Section 13.2.2 are required to be tested in accordance with Section 1708.4.
3. Architectural, mechanical and electrical components in structures assigned to Seismic Design Category C, D, E or F with an Ip equal to 1.0 are required to be tested in accordance with Section 1708.4 where the general design requirements of ASCE 7 Section 13.2.1, Item 2 for manufacturer’s certification are satisfied by testing.
4. The seismic isolation system in seismically isolated structures shall meet the testing requirements of Section 1708.5.

1708.2 Concrete reinforcement. Where reinforcement complying with ASTM A 615 is used to resist earthquake-induced flexural and axial forces in special moment frames, special structural walls and coupling beams connecting special structural walls, in structures assigned to Seismic Design Category B, C, D, E or F as determined in Section 1613, the reinforcement shall comply with Section 21.1.5.2 of ACI 318. Certified mill test reports shall be provided for each shipment of such reinforcement. Where reinforcement complying with ASTM A 615 is to be welded, chemical tests shall be performed to determine weldability in accordance with Section 3.5.2 of ACI 318.

1708.3 Structural steel. Testing for structural steel shall be in accordance with the quality assurance plan requirements of AISC 341.

Exceptions:

1. Testing for structural steel in structures assigned to Seismic Design Category C that are not specifically detailed for seismic resistance, with a response modification coefficient, R, of 3 or less, excluding cantilever column systems.
2. For ordinary moment frames, ultrasonic and magnetic particle testing of complete joint penetration groove welds are only required for demand critical welds.

1708.4 Seismic certification of nonstructural components. The registered design professional shall state the applicable seismic certification requirements for nonstructural components and designated seismic systems on the construction documents.

1. The manufacturer of each designated seismic system components subject to the provisions of ASCE 7 Section 13.2.2 shall test or analyze the component and its anchorage and submit a certificate of compliance for review and acceptance by the registered design professional responsible for the design of the designated seismic system and for approval by the building official. Certification shall be based on an actual test on a shake table, by three-dimensional shock tests, by an analytical method using dynamic characteristics and forces, or by the use of experience data (i.e., historical data demonstrating acceptable seismic performance) or by more rigorous analysis providing for equivalent safety.
2. Manufacturer’s certification of compliance for the general design requirements of ASCE 7 Section 13.2.1 shall be based on analysis, testing or experience data.

1708.5 Seismically isolated structures. For required system tests, see Section 17.8 of ASCE 7.
SECTION 1709
CONTRACTOR RESPONSIBILITY

1709.1 Contractor responsibility. Each contractor responsible for the construction of a main wind- or seismic-force-resisting system, designated seismic system or a wind- or seismic-resisting component listed in the statement of special inspections shall submit a written statement of responsibility to the building official and the owner prior to the commencement of work on the system or component. The contractor’s statement of responsibility shall contain acknowledgement of awareness of the special requirements contained in the statement of special inspection.

SECTION 1710
STRUCTURAL OBSERVATIONS

1710.1 General. Where required by the provisions of Section 1710.2 or 1710.3, the owner shall employ a registered design professional to perform structural observations as defined in Section 1702.

Prior to the commencement of observations, the structural observer shall submit to the building official a written statement identifying the frequency and extent of structural observations.

At the conclusion of the work included in the permit, the structural observer shall submit to the building official a written statement that the site visits have been made and identify any reported deficiencies which, to the best of the structural observer’s knowledge, have not been resolved.

1710.2 Structural observations for seismic resistance. Structural observations shall be provided for those structures assigned to Seismic Design Category D, E or F, as determined in Section 1613, where one or more of the following conditions exist:

1. The structure is classified as Occupancy Category III or IV in accordance with Table 1604.5.
2. The height of the structure is greater than 75 feet (22 860 mm) above the base.
3. The structure is assigned to Seismic Design Category E, is classified as Occupancy Category I or II in accordance with Table 1604.5, and is greater than two stories above grade plane.
4. When so designated by the registered design professional responsible for the structural design.
5. When such observation is specifically required by the building official.

1710.3 Structural observations for wind requirements. Structural observations shall be provided for those structures sited where the basic wind speed exceeds 110 mph (49 m/sec) determined from Figure 1609, where one or more of the following conditions exist:

1. The structure is classified as Occupancy Category III or IV in accordance with Table 1604.5.
2. The building height of the structure is greater than 75 feet (22 860 mm).
3. When so designated by the registered design professional responsible for the structural design.
4. When such observation is specifically required by the building official.

SECTION 1711
DESIGN STRENGTHS OF MATERIALS

1711.1 Conformance to standards. The design strengths and permissible stresses of any structural material that are identified by a manufacturer's designation as to manufacture and grade by mill tests, or the strength and stress grade is otherwise confirmed to the satisfaction of the building official, shall conform to the specifications and methods of design of accepted engineering practice or the approved rules in the absence of applicable standards.

1711.2 New materials. For materials that are not specifically provided for in this code, the design strengths and permissible stresses shall be established by tests as provided for in Section 1712.

SECTION 1712
ALTERNATIVE TEST PROCEDURE

1712.1 General. In the absence of approved rules or other approved standards, the building official shall make, or cause to be made, the necessary tests and investigations; or the building official shall accept duly authenticated reports from approved agencies in respect to the quality and manner of use of new materials or assemblies as provided for in Section 104.11. The cost of all tests and other investigations required under the provisions of this code shall be borne by the applicant.

SECTION 1713
TEST SAFE LOAD

1713.1 Where required. Where proposed construction is not capable of being designed by approved engineering analysis, or where proposed construction design method does not comply with the applicable material design standard, the system of construction or the structural unit and the connections shall be subjected to the tests prescribed in Section 1715. The building official shall accept certified reports of such tests conducted by an approved testing agency, provided that such tests meet the requirements of this code and approved procedures.

SECTION 1714
IN-SITU LOAD TESTS

1714.1 General. Whenever there is a reasonable doubt as to the stability or load-bearing capacity of a completed building, structure or portion thereof for the expected loads, an engineering assessment shall be required. The engineering assessment shall involve either a structural analysis or an in-situ load test, or both. The structural analysis shall be based on actual material properties and other as-built conditions that affect stability or load-bearing capacity, and shall be conducted in accordance with the applicable design standard. If the structural assessment determines that the load-bearing capacity is less than that.
required by the code, load tests shall be conducted in accordance with Section 1714.2. If the building, structure or portion thereof is found to have inadequate stability or load-bearing capacity for the expected loads, modifications to ensure structural adequacy or the removal of the inadequate construction shall be required.

1714.2 Test standards. Structural components and assemblies shall be tested in accordance with the appropriate material standards listed in Chapter 35. In the absence of a standard that contains an applicable load test procedure, the test procedure shall be developed by a registered design professional and approved. The test procedure shall simulate loads and conditions of application that the completed structure or portion thereof will be subjected to in normal use.

1714.3 In-situ load tests. In-situ load tests shall be conducted in accordance with Section 1714.3.1 or 1714.3.2 and shall be supervised by a registered design professional. The test shall simulate the applicable loading conditions specified in Chapter 16 as necessary to address the concerns regarding structural stability of the building, structure or portion thereof.

1714.3.1 Load test procedure specified. Where a standard listed in Chapter 35 contains an applicable load test procedure and acceptance criteria, the test procedure and acceptance criteria in the standard shall apply. In the absence of specific load factors or acceptance criteria, the load factors and acceptance criteria in Section 1714.3.2 shall apply.

1714.3.2 Load test procedure not specified. In the absence of applicable load test procedures contained within a standard referenced by this code or acceptance criteria for a specific material or method of construction, such existing structure shall be subjected to a test procedure developed by a registered design professional that simulates applicable loading and deformation conditions. For components that are not a part of the seismic-force-resisting system, the test load shall be equal to two times the unfactored design loads. The test load shall be left in place for a period of 24 hours. The structure shall be considered to have successfully met the test requirements where the following criteria are satisfied:

1. Under the design load, the deflection shall not exceed the limitations specified in Section 1604.3.
2. Within 24 hours after removal of the test load, the structure shall have recovered not less than 75 percent of the maximum deflection.
3. During and immediately after the test, the structure shall not show evidence of failure.

SECTION 1715
PRECONSTRUCTION LOAD TESTS

1715.1 General. In evaluating the physical properties of materials and methods of construction that are not capable of being designed by approved engineering analysis or do not comply with applicable material design standards listed in Chapter 35, the structural adequacy shall be predetermined based on the test load criteria established in this section.

1715.2 Load test procedures specified. Where specific load test procedures, load factors and acceptance criteria are included in the applicable design standards listed in Chapter 35, such test procedures, load factors and acceptance criteria shall apply. In the absence of specific test procedures, load factors or acceptance criteria, the corresponding provisions in Section 1715.3 shall apply.

1715.3 Load test procedures not specified. Where load test procedures are not specified in the applicable design standards listed in Chapter 35, the load-bearing and deformation capacity of structural components and assemblies shall be determined on the basis of a test procedure developed by a registered design professional that simulates applicable loading and deformation conditions. For components and assemblies that are not a part of the seismic-force-resisting system, the test shall be as specified in Section 1715.3.1. Load tests shall simulate the applicable loading conditions specified in Chapter 16.

1715.3.1 Test procedure. The test assembly shall be subjected to an increasing superimposed load equal to not less than two times the superimposed design load. The test load shall be left in place for a period of 24 hours. The test assembly shall be considered to have successfully met the test requirements if the assembly recovers not less than 75 percent of the maximum deflection within 24 hours after the removal of the test load. The test assembly shall then be reloaded and subjected to an increasing superimposed load until either structural failure occurs or the superimposed load is equal to two and one-half times the load at which the deflection limitations specified in Section 1715.3.2 were reached, or the load is equal to two and one-half times the superimposed design load. In the case of structural components and assemblies for which deflection limitations are not specified in Section 1715.3.2, the test specimen shall be subjected to an increasing superimposed load until structural failure occurs or the load is equal to two and one-half times the desired superimposed design load. The allowable superimposed design load shall be taken as the lesser of:

1. The load at the deflection limitation given in Section 1715.3.2.
2. The failure load divided by 2.5.
3. The maximum load applied divided by 2.5.

1715.3.2 Deflection. The deflection of structural members under the design load shall not exceed the limitations in Section 1604.3.

1715.4 Wall and partition assemblies. Load-bearing wall and partition assemblies shall sustain the test load both with and without window framing. The test load shall include all design load components. Wall and partition assemblies shall be tested both with and without door and window framing.

1715.5 Exterior window and door assemblies. The design pressure rating of exterior windows and doors in buildings shall be determined in accordance with Section 1715.5.1 or 1715.5.2.

Exception: Structural wind load design pressures for window units smaller than the size tested in accordance with Section 1715.5.1 or 1715.5.2 shall be permitted to be higher than the design value of the tested unit provided such higher...
pressures are determined by accepted engineering analysis. All components of the small unit shall be the same as the tested unit. Where such calculated design pressures are used, they shall be validated by an additional test of the window unit having the highest allowable design pressure.

1715.5.1 Exterior windows and doors. Exterior windows and sliding doors shall be tested and labeled as conforming to AAMA/WDMA/CSA101/LS.2/A440. The label shall state the name of the manufacturer, the approved labeling agency and the product designation as specified in AAMA/WDMA/CSA101/LS.2/A440. Exterior side-hinged doors shall be tested and labeled as conforming to AAMA/WDMA/CSA101/LS.2/A440 or comply with Section 1715.5.2. Products tested and labeled as conforming to AAMA/WDMA/CSA101/LS.2/A440 shall not be subject to the requirements of Sections 2403.2 and 2403.3.

1715.5.2 Exterior windows and door assemblies not provided for in Section 1715.5.1. Exterior window and door assemblies shall be tested in accordance with ASTM E 330. Structural performance of garage doors shall be determined in accordance with either ASTM E 330 or ANSI/DASMA 108, and shall meet the acceptance criteria of ANSI/DASMA 108. Exterior window and door assemblies containing glass shall comply with Section 2403. The design pressure for testing shall be calculated in accordance with Chapter 16. Each assembly shall be tested for 10 seconds at a load equal to 1.5 times the design pressure.

1715.6 Test specimens. Test specimens and construction shall be representative of the materials, workmanship and details normally used in practice. The properties of the materials used to construct the test assembly shall be determined on the basis of tests on samples taken from the load assembly or on representative samples of the materials used to construct the load test assembly. Required tests shall be conducted or witnessed by an approved agency.

SECTION 1716
MATERIAL AND TEST STANDARDS

1716.1 Test standards for joist hangers and connectors.

1716.1.1 Test standards for joist hangers. The vertical load-bearing capacity, torsional moment capacity and deflection characteristics of joist hangers shall be determined in accordance with ASTM D 1761 using lumber having a specific gravity of 0.49 or greater, but not greater than 0.55, as determined in accordance with AF&PA NDS for the joist and headers.

Exception: The joist length shall not be required to exceed 24 inches (610 mm).

1716.1.2 Vertical load capacity for joist hangers. The vertical load capacity for the joist hanger shall be determined by testing a minimum of three joist hanger assemblies as specified in ASTM D 1761. If the ultimate vertical load for any one of the tests varies more than 20 percent from the average ultimate vertical load, at least three additional tests shall be conducted. The allowable vertical load of the joist hanger shall be the lower value determined from the following:

1. The lowest ultimate vertical load for a single hanger from any test divided by three (where three tests are conducted and each ultimate vertical load does not vary more than 20 percent from the average ultimate vertical load).
2. The average ultimate vertical load for a single hanger from all tests divided by three (where six or more tests are conducted).
3. The average from all tests of the vertical loads that produce a vertical movement of the joist with respect to the header of 1/8 inch (3.2 mm).
4. The sum of the allowable design loads for nails or other fasteners utilized to secure the joist hanger to the wood members and allowable bearing loads that contribute to the capacity of the hanger.
5. The allowable design load for the wood members forming the connection.

1716.1.3 Torsional moment capacity for joist hangers. The torsional moment capacity for the joist hanger shall be determined by testing at least three joist hanger assemblies as specified in ASTM D 1761. The allowable torsional moment of the joist hanger shall be the average torsional moment at which the lateral movement of the top or bottom of the joist with respect to the original position of the joist is 1/8 inch (3.2 mm).

1716.1.4 Design value modifications for joist hangers. Allowable design values for joist hangers that are determined by Item 4 or 5 in Section 1716.1.2 shall be permitted to be modified by the appropriate duration of loading factors as specified in AF&PA NDS but shall not exceed the direct loads as determined by Item 1, 2 or 3 in Section 1716.1.2. Allowable design values determined by Item 1, 2 or 3 in Section 1716.1.2 shall not be modified by duration of loading factors.

1716.2 Concrete and clay roof tiles.

1716.2.1 Overturning resistance. Concrete and clay roof tiles shall be tested to determine their resistance to overturning due to wind in accordance with SBCCI SSTD 11 and Chapter 15.

1716.2.2 Wind tunnel testing. When roof tiles do not satisfy the limitations in Chapter 16 for rigid tile, a wind tunnel test shall be used to determine the wind characteristics of the concrete or clay tile roof covering in accordance with SBCCI SSTD 11 and Chapter 15.